



Approval body for construction products and types of construction

Bautechnisches Prüfamt

An institution established by the Federal and Laender Governments



European Technical Assessment

ETA-06/0009 of 2 June 2017

English translation prepared by DIBt - Original version in German language

General Part

Technical Assessment Body issuing the European Technical Assessment:

Trade name of the construction product

Product family to which the construction product belongs

Manufacturer

Manufacturing plant

This European Technical Assessment contains

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

This version replaces

Deutsches Institut für Bautechnik

Binderholz Brettsperrholz BBS

Solid wood slab element to be used as a structural element in buildings

Binderholz Bausysteme GmbH Zillertalstraße 39 6263 FÜGEN ÖSTERREICH

W01, W02, W03, W04

26 pages including 6 annexes which form an integral part of this assessment

European Assessment Document (EAD) 130005-00-0304

ETA-06/0009 issued on 7 April 2016



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Z760.17 8.03.04-37/16



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Specific Part

Technical description of the product 1

"Binderholz Brettsperrholz BBS" are plane timber building components which are made of at least three layers of softwood boards. Adjacent layers are glued together with an angle of 90°. The cross section of the elements is symmetric¹.

The components and the system setup of the elements are given in Annex 1. The building elements are plane.

Two adjacent layers may be oriented with parallel grain direction if a symmetric and crosswise blocked structure is guaranteed.

Not load-bearing outer layers are permissible.

Single board layers (maximum 50% of the cross section) may be replaced by one- and multilayer solid wood panels. The solid wood panels shall be suitable for structural use.

The elements can be produced with a width up to 3.5 m and a length up to 22 m as Großformat and with a width up to 1.25 m und a length up to 5 m as Systemformat.

The building components in Systemformat with a width up to 1.25 m are connected in the plant in longitudinal direction by large finger jointing in accordance with EN 14080 to a length of up to 24 m.

The cross laminated timber elements are manufactured using the automated manufacturing process in accordance with the technical documentation and inspection.

The layers are bonded together to the required thickness of the cross laminated timber.

Specifications of the used boards are given in Annex 2. Boards are visually or machine strength graded. Only technically dried wood is used.

Only boards which are planed on both sides of the outer layer are used. The boards may be connected by finger joints in longitudinal direction according to EN 14080. There are no butt joints.

The single boards of the layers in longitudinal direction may be glued at narrow side. The maximum width of the gap is given in Annex 2.

The solid wood slab elements correspond to the specifications given in Annexes 1 to 3 of this European Technical Assessment. The material characteristics, dimensions and tolerances of the solid wood slab elements not indicated in these Annexes are given in the technical documentation of the European Technical Assessment.

Specification of the intended use in accordance with the applicable European 2 **Assessment Document**

The elements are intended to be used as load-bearing and/or stiffening or not load-bearing wall, ceiling/floor, roof and special construction components for timber structures. For the taking up and transmitting of loads they may be stressed both perpendicular to the element plane and in the element plane.

The solid wood slab element shall be subjected to static and quasi-static actions only.

The solid wood slab element is intended to be used in service classes 1 and 2 according to EN 1995-1-1.

Members shall be provided with an effective protection for the solid wood slab elements in service.

For regulations on deviations from symmetry see Annex 2



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The performances given in Section 3 are only valid if the solid wood slab elements are used in compliance with the specifications and conditions given in Annex 1 to 5.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the solid wood slab element of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

Design

The suitability of the solid wood slab elements for the specified purpose is given under the following conditions:

- Design of the solid wood slab elements is carried out under the responsibility of an engineer experienced in such products.
- Design of the works shall account for the protection of the solid wood slab elements.
- The solid wood slab elements are installed correctly.

The design of the solid wood slab element can be performed according to EN 1995-1-1, taking into account Annexes 2 to 5 of the European Technical Assessment. Standards and regulations valid in the place of use shall be considered.

Packaging, transport, storage, maintenance and repair

The solid wood slab elements shall be protected during transport and storage against any damage and detrimental moisture effects. The manufacturer's instructions for packaging, transport and storage shall be observed.

The assessment of the fitness for use is based on the assumption that maintenance is not required during the assumed intended working life. In case of a severe damage of a solid wood slab element immediate actions regarding the mechanical resistance and stability of the works shall be initiated. Should this situation arise replacement of the elements can be necessary.

Installation

The manufacturer shall prepare assembling instructions in which the product-specific characteristics and important measures to be taken into consideration for assembling are described. The assembling instructions shall be available at every construction site.

The assembling of the solid wood slab elements according to this European Technical Assessment shall be carried out by appropriately qualified personnel.

Elements which are directly exposed to the weather shall be provided with an effective protection for the cross laminated timber element during assembling and service.

The safety-at-work and health protection regulations have to be observed.

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3 Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability ¹⁾ (BWR 1)

Performance
Annex 3

¹⁾ This characteristic also relates to BWR 4.

For gluing the board layers, for the finger joint connection of the individual boards and for the large finger joint connection an adhesive which meet the requirements of EN 301 shall be used. Alternatively a one component polyurethane adhesive which meets the requirements of EN 15425 and EN 14080, annex B.2 considering annex B.1, may be used.

Regarding the applicable type of adhesive national regulations apply.²

Details on the adhesives and the bonding process are deposited with Deutsches Institut für Bautechnik.

3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance	
Reaction to fire	Annex 3	
Resistance to fire	Annex 3	

²⁾ Load bearing capacity and stiffness regarding mechanical actions perpendicular to and in plane of the solid wood slab element.



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3.3 Hygiene, health and the environment (BWR 3)

Essential characteristic	Performance
Content of dangerous substances	The manufacturer has submitted a written declaration to the Technical Assessment Body (DIBt) that no dangerous substances > 0.1 wt. % are used in the product assessed by the present ETA. The chemical composition of the adhesives for gluing the board layers, the finger joint connection of the individual boards and the universal finger joint connection has to be in compliance with the chemical composition deposited at the Technical Assessment Body (DIBt).
Formaldehyde emission	The manufactured cross-laminated timber and the used wood based panels comply which EN 13986 of formaldehyde class E1.
Wood preservatives or flame retardants	Wood preservatives and flame retardants are not subject of the ETA.
Release scenarios regarding BWR 3	IA 1, IA 2
Water vapour permeability - Water vapour transmission	Annex 3

3.4 Safety and accessibility in use (BWR 4)

Essential characteristic	Performance
Impact resistance	Annex 3

3.5 Protection against noise (BWR 5)

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Essential characteristic	Performance	
Airborne sound insulation	no performance assessed	
Impact sound insulation	no performance assessed	
Sound absorption	no performance assessed	

3.6 Energy economy and heat retention (BWR 6)

Essential characteristic	Performance
Thermal conductivity	Annex 3
Air permeability	no performance assessed
Thermal inertia	Annex 3

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Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with EAD No. 130005-00-0304 the applicable European legal act is: 1997/176/EC amended by 2001/596/EC

The system to be applied is: 1

Technical details necessary for the implementation of the AVCP system, as provided for in the applicable EAD

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited with Deutsches Institut für Bautechnik.

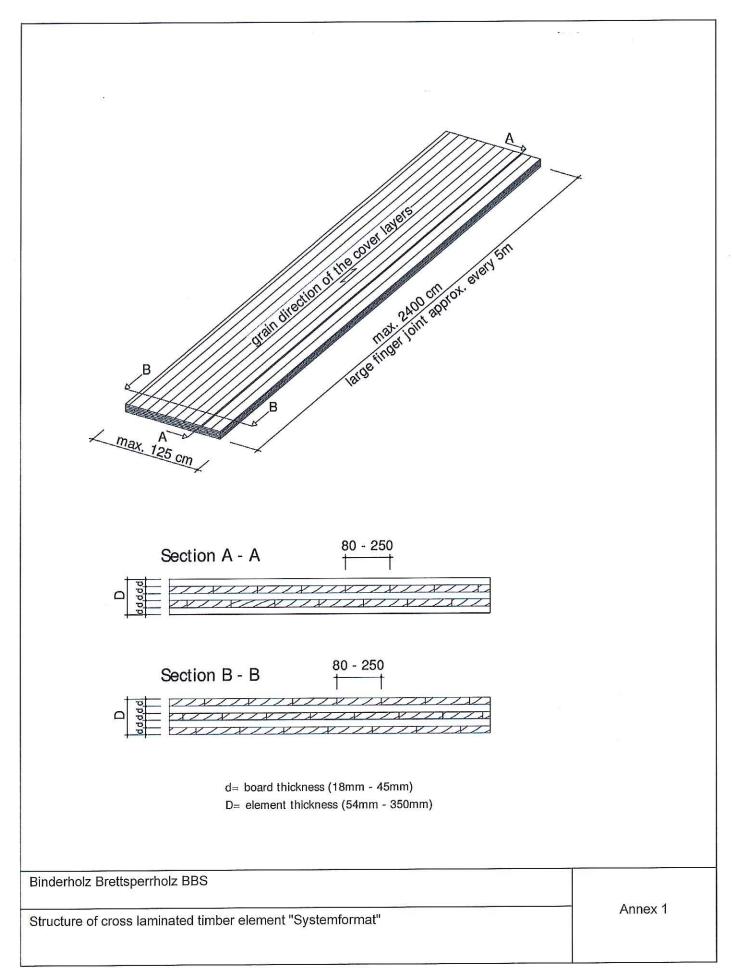
Issued in Berlin on 2 June 2017 by Deutsches Institut für Bautechnik

BD Dipl.-Ing. Andreas Kummerow Head of Department

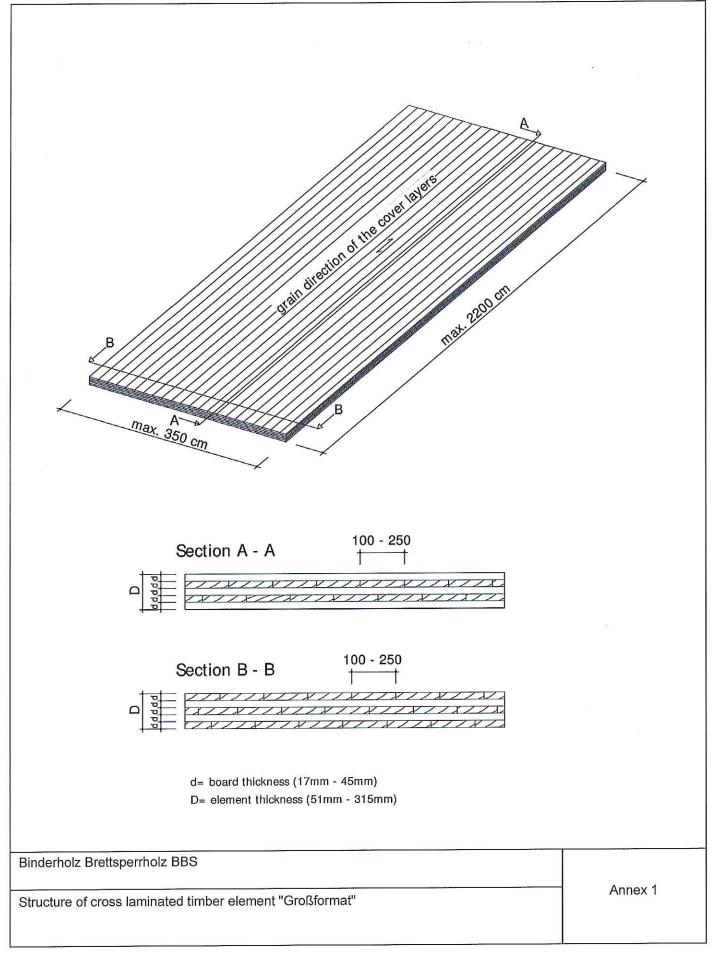
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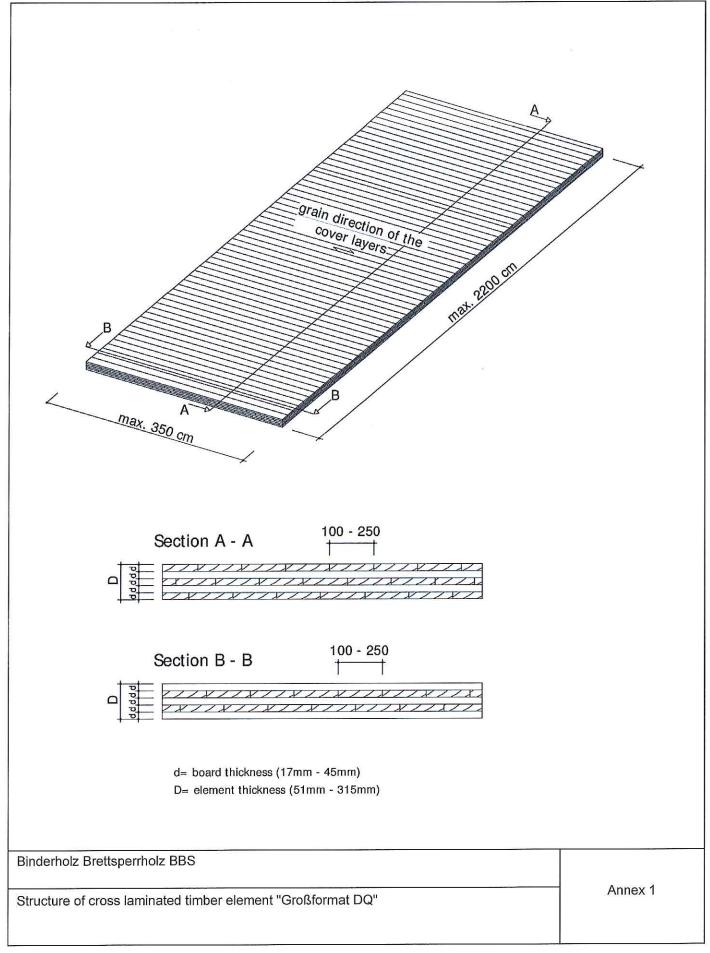














Binderholz Brettsperrholz BBS "Systemformat" Characteristic Specification	
	Specification
Cross laminated timber element	
Thickness	54 to 350 mm
Tolerance in thickness	± 1 mm
Width	≤ 1.25 m
Tolerance in width	± 2 mm
Length	≤ 5 m
Tolerance in length (relating to a max. length up to 5 m)	± 2 mm
Length of the element with large finger joint	≤ 24 m
Number of layers	3 ≤ n ≤ 9
maximum number of consecutive layers having the same grain direction	≤ 2
maximum width of gap between the boards of a layer	4 mm
Large finger joints	according to EN 14080
Layup	Symmetric layup 1)
Boards	
Material	softwood
Strength class according to EN 338 Cover layers / longitudinal layers (having the same grain direction as cover layers) Cross layer (having the a grain direction perpendicular to the cover layer)	≥ 90 % C24; < 10 % C16 ²⁾ ≥ 30 % C24; < 70 % C16 ³⁾
Thickness	18 to 45 mm
Width	80 to 250 mm
Ratio width to thickness of the cross-layers	≥ 4:1
Moisture of wood according to EN 13183-2	10 ± 2 % 12 ± 2 % Within one cross laminated timber element only one of the specified moisture ranges shall be applied.
Finger joints	according to EN 14080
Wood based panels	
Material	Solid wood panels according to EN 13986
Thickness	12 to 60 mm
Joints	Joints perpendicular to the longitudinal direction are not allowed. Joints parallel to the longitudinal
	direction shall be taken into account in the design.

Binderholz Brettsperrholz BBS	
Dimensions and specifications of the cross laminated timber	Annex 2



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Binderholz Brettsperrholz BBS "Großformat" and "Großformat DQ"	
Characteristic	Specification
Cross laminated timber element	
Thickness	51 to 315 mm
Tolerance in thickness	± 1 mm
Width	≤ 3.5 m
Tolerance in width	± 2 mm
Length	≤ 22 m
Tolerance in length (relating to a max. length up to 22 m)	± 2 mm
Number of layers	3 ≤ n ≤ 7
maximum number of consecutive layers having the same grain direction	≤ 2
maximum width of gaps between the boards of a layer	4 mm
Layup	Symmetric layup 1)
Boards	
Material	softwood
Strength class according to EN 338 Cover layers / longitudinal layers (having the same grain direction as cover layers) Cross layers (having the grain direction perpendicular to the cover layers)	≥ 90 % C24; < 10 % C16 ²⁾
Thickness	17 to 45 mm
Width	100 to 250 mm
Ratio width to thickness of the cross-layers	≥ 4:1
Moisture of wood according to EN 13183-2	10 ± 2 % 12 ± 2 % Within one cross laminated timbe element only one of the specified moisture ranges shall be applied.
Finger joints	according EN 14080 mechanical resistance
Wood based panels	
Material	Solid wood panels according to EN 13986
Thickness	12 to 60 mm
Joints	Joints perpendicular to the longitudinal direction are not allowed. Joints parallel to the longitudinal
	direction shall be taken into account in the design.

inderholz Brettsperrholz BBS	
imensions and specifications of the cross laminated timber	Annex 2



Table 1 (continued)

- 1) Deviations from symmetric layup:
 - The layup (cross-section) of the element is symmetric with respect to the centre layer.
 - When using boards of different strength classes the deviations of the elastic centre of gravity from the geometric centre may be disregarded.
 - The layup may also be considered as symmetrical when a thick top layer is substituted by two thinner fibre-parallel layers with approximately the same overall thickness.
 - Layers acc. to EN 13986 arranged in addition to the symmetric layup shall be disregarded in design.
- Deviations from symmetry caused by load bearing multi-layered solid wood panels acc. to EN 13986 have to be considered if necessary.

2)	The	proportion	of wood	of grade	C16 may	be disregard	ed by wa	ay of calcul	ation.
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Binderholz Brettsperrholz BBS	
Dimensions and specifications of the cross laminated timber	Annex 2
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Table 2: Essential requirements of the multilayered timber elements

BWR	Requirement Ve	rific	ation method	Class / Us	e category / Value		
DILIK!	Mechanical resistance and	3,000,000,000		I ANTI PER TORNIE DE			
	For the calculation of the individual layers the characteristic strength and stiffness values of softwood of the corresponding strength classes acc. to EN 338 shall be used taking into consideration the definitions in Annex 2. In addition the following values apply:						
	Mechanical actions in plane of cross laminated timber		Modulus of elasticity parallel to the grain of the boards	E _{0,mean}	12.000 N/mm²		
			Modulus of elasticity parallel to the grain of the boards	E _{0,mean}	12.000 N/mm²		
	Mechanical actions perpendicular to the plane of cross laminated timber		Rolling shear strength "Systemformat" "Großformat" and "Großformat DQ" (5%-fractile)	f _{v,9090,k}	1.0 N/mm²		
1			Rolling shear modulus (mean value)	G _{9090,mean}	50 N/mm²		
	In case of connecting the elements by large finger joints the characteristic bending strength is to reduce by 25 %. In case of tensile stresses in the panel plane the characteristic tensile strength is to reduce by 30 %. For references regarding the calculation see annex 4. National regulations might have to be followed.						
	Creep and duration of load according to EN 1995-1-1						
	Dimensional stability	Moi	sture content during use shall erse deformations can occur.	not change	to such extent that		
	In-service environment		EN 1995-1-1		2		
	Bond integrity		EAD 130005-00-0304		Passed		
	Safety in case of fire						
	Reaction to fire						
2	Timber elements except for floorings		Commission Decision 2005/610/EC		Euroclass D-s2, d0		
	Resistance to fire						
	Charring rate	EN 1995-1-2		0.7 mm/min			
	Hygiene, health and the env	/iron	ment				
3	Water vapour permeability μ	EN	ISO 10456	20 to 50			
	Content of dangerous substances		EAD 130005-00-0304 See clau		ause 3		

2	Annex 3
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	Safety in use					
4	Impact resistance Soft body resistance is assumed to be fulfilled for walls with a minimum of 3 layers and minimum thickness of 60 mm.					
	Protection against noise					
e e	Airbourne sound insulation	no performance assessed				
5	Impact sound insulation	no performance assessed				
	Sound absorption	no performance assessed				
	Energy economy and heat retention					
6	Thermal conductivity λ	EN ISO 10456	0,12 W/(m ² · K)			
	Air tightness	no performance assessed				
	Thermal inertia c _p	EN ISO 10456	1.600 J/(kg · K)			

3



1 Recommendations for the design of the elements

1.1 General

Design, calculation and realization may be performed according to EN 1995-1-1 taking into account the following provisions. For the calculation according to EN 1995-1-1 national regulations may have to be followed.

The determination of the distribution of stresses and internal forces must consider the influence of shear deformations of the cross layers. In Annex 6 advice is given on how to perform the calculation of the elements.

If using panels as cover, the deformation of the covers might have to be taken into account. These cover layers may not be used for calculation of the bearing capacity of the cross laminated timber elements.

1.2 Characteristic values

The characteristic strength and stiffness values can be taken from Annex 2 and 3. In addition the following applies:

The deformations caused by shear forces may be calculated by using the element thickness D irrespective of the given layup and a global shear modulus of $G = 60 \text{ N/mm}^2$ for 3-layer elements and of $G = 80 \text{ N/mm}^2$ for elements with 5 layers or more.

1.3 Mechanical actions perpendicular to the element's plane

1.3.1 Bending and shear

For the calculation of the characteristic values of the element according to Annex 6, only the boards, which are oriented parallel to the span direction, may be considered.

For the verification of the bending strength of a layer the design value of the bending strength may be multiplied with a system factor k_a:

$$k_{\ell} = min \begin{cases} 1 + 0.025 \text{ n} \\ 1.1 \end{cases}$$

where n = number of adjacent boards

1.3.2 Tension and compression

The behaviour in bearing and deformation against compression perpendicular to the element's plane can be calculated according to EN 1995-1-1 using the strength and stiffness values given in chapter 1.2. Tension loads perpendicular to plane of the element should be avoided.

1.4 Mechanical actions in plane of the element

For loads in plane of the element only layers can be taken into account, where the direction of the grain is parallel to the stresses occurring from external loads.

Binderholz Brettsperrholz BBS	
Design considerations	Annex 4



1.4.1 Shear

Shear stresses may be calculated with the gross cross section. These shear stresses are to be compared with an effective characteristic shear strength $f_{v,k}$ according to the following equation:

$$f_{v,k} = min \begin{cases} 3.5 \\ 8.0 \frac{D_{net}}{D} \\ 2.5 \frac{(n-1)(a^2 + b^2)}{6 D b} \end{cases}$$
 in [N/mm²]

where

D element thickness (see Annex 1)

D_{net} total thickness of longitudinal or cross layers within the element; the smaller value applies

n number of layers within the element, adjacent layers with parallel lamellae shall be considered as one layer and

a, b width of the boards in the longitudinal or cross layers, where b > a (If a and b is unknown, the minimum value must be applied for b.)

1.4.2 Tension and compression

The load-bearing and deformation behaviour in the element plane can be calculated according to EN 1995-1-1 using the strength and stiffness values given in chapter 1.2.

1.5 Buckling

For buckling the 5%-quantile values of the modulus of elasticity may be set to 5/6 of the corresponding mean value: $E_{0.05} = 5/6 E_{0,mean}$

The imperfection factor β_c may be set to $\beta_c = 0.1$ as for glued laminated timber.

Binderholz Brettsperrholz BBS

Design considerations

Annex 4

Design according to the theory of flexible bonded beams

The calculation of elements with up to five layers can be performed using the theory of flexible bonded beams as described in EN 1995-1-1.

To consider deformations due to shear the factor si/Ki according to the standard is substituted by the factor $h_i/(G_R \cdot b)$

The effective moment of inertia is calculated by:

$$I_{ef} = \sum_{i=1}^{3} (I_i \cdot + \gamma_i \cdot A_i \cdot a_i^2) \qquad \text{where} \quad A_i = b_i \cdot h_i \, ; \qquad \qquad I_i = \frac{b_i \cdot h_i^3}{12}$$

$$I_i = \frac{b_i \cdot h_i^3}{12}$$

$$\gamma_1 = \frac{1}{1 + \frac{\pi^2 \cdot E_0 \cdot A_1 \cdot \overline{h_1}}{G_R \cdot b \cdot l^2}}; \qquad \gamma_2 = 1; \qquad \qquad \gamma_3 = \frac{1}{1 + \frac{\pi^2 \cdot E_0 \cdot A_3 \cdot \overline{h_2}}{G_R \cdot b \cdot l^2}}$$

$$\gamma_2 = 1$$
;

$$\gamma_3 = \frac{1}{1 + \frac{\pi^2 \cdot E_0 \cdot A_3 \cdot \overline{h_2}}{G_R \cdot b \cdot l^2}}$$

$$a_1 = \left(\frac{h_1}{2} + \overline{h_1} + \frac{h_2}{2}\right) - a_2$$
;

$$a_3 = \left(\frac{h_2}{2} + \overline{h_2} + \frac{h_3}{2}\right) + a_2$$

$$a_2 = \frac{\gamma_1 \cdot A_1 \cdot \left(\frac{h_1}{2} + \overline{h_1} + \frac{h_2}{2}\right) - \gamma_3 \cdot A_3 \cdot \left(\frac{h_2}{2} + \overline{h_2} + \frac{h_3}{2}\right)}{\sum\limits_{\substack{j=1 \\ i=1}}^3 (\gamma_i \cdot A_i)}$$

The bending stress in the centre of the boards may be disregarded.

The governing bending stress in the outermost fibre of the boards:

$$\sigma_{m,r,i,d} = \pm \frac{M_d}{I_{ef}} \cdot \left(\gamma_i \cdot a_i + \frac{h_i}{2} \right) \le f_{m,d}$$

Shear design is in the governing plane:

$$\tau_{v,d} = \frac{V_d \cdot \gamma_i \cdot S_i}{I_{ef} \cdot b} \le f_{R,d}$$

Notation:

 $h_{tot} =$ thickness of the whole element [mm]

thickness of the layer i parallel to the direction of load transfer [mm]

thickness of the layer i perpendicular to the direction of load transfer [mm]

width of the element [mm]

number of layers

span width [mm]

effective moment of inertia [Nmm2]

rolling shear modulus [N/mm²] G_R

modulus of elasticity parallel to the grain of the boards [N/mm²] E_0

Binderholz Brettsperrholz BBS

Design considerations

Annex 4



2 Recommendations for the design of the fasteners

2.1 General

The determination of characteristic values of the load-bearing capacity of fasteners in the element shall be carried out according to EN 1995-1-1 or acc. to a European Technical Assessment which has been granted for the relevant fastener as for softwood or for glued laminated timber. For the calculation according to European regulations national provisions may apply.

Wide faces are the surfaces of the element parallel to the plane of the element consisting of the surface of the outer layers.

Narrow faces are the lateral and the cross grain board surfaces perpendicular to the plane of the element.

Only fasteners according to EN 1995-1-1 or a European Technical Approval or Assessment or according to national regulations may be used.

The grain direction of the cover layers governs the minimum spacings of the fasteners as well as the embedding strength is.

For the design of the fasteners the values acc. to Table 3 shall be assumed:

Table 3: Density values

purpose of use	symbol	Systemformat Großformat Großformat DQ
self-weight BBS	YG,k	4,5 kN/m ³
calculation of the connection stiffness of - screws, nails and staples - bolts and dowels in the wide face - split ring, shear plate and toothed-plate connectors in the wide face - split ring and shear plate connectors in the narrow face	P _{mean}	420 kg/m³
calculation of load carrying capacity - screws, nails and staples - split ring, shear plate and toothed-plate connectors in the wide face - bolts and dowels in the wide face - split ring and shear plate connectors in the narrow face	Pk	350 kg/m³

Annex 5

2.2 Connections with dowels and bolts

The characteristic load-carrying capacity of dowelled or bolted connections in the wide faces is to be determined with the embedding strength according to the following equation:

$$f_{h,\alpha,k} = \frac{32 \cdot (1 - 0.015 \cdot d)}{1.1 \cdot \sin^2 \alpha + \cos^2 \alpha}$$
 in N/mm²

where

d fastener diameter in mm

angle between force and grain direction of the cover layer

The grain direction of the cover layers is taken into account for the calculation of the embedding strength.

The characteristic load-carrying capacity of dowelled or bolted connections in the narrow faces is to be determined with the embedding strength according to the following equation:

$$f_{h,k} = 9 \cdot (1 - 0.017 \cdot d)$$
 in N/mm²

For dowels with a diameter of \geq 10 mm in the wide faces, $n_{ef} = n$ may be assumed.

2.3 Nails

General

The minimum diameter for nails in Binderholz Brettsperrholz BBS Systemformat und Binderholz Brettsperrholz BBS Großformat is 2.8 mm.

Laterally loaded nails - wide faces

The characteristic load-carrying capacity of laterally loaded nails in the wide faces is to be determined according to EN 1995-1-1. The embedding strength shall be calculated with the characteristic density of the boards of the outer layer.

The effective number of nails n_{ef} may be set equal to the actual number n.

Laterally loaded nails - narrow faces

Nails in the narrow faces of the elements shall not be considered as load-bearing.

Axially loaded nails

Only threaded nails with a characteristic withdrawal parameter of $f_{ax,k} \ge 50 \cdot 10^{-6} \cdot \rho_k^2$ and a characteristic pull through parameter $f_{head,k} \ge 100 \cdot 10^{-6} \cdot \rho_k^2$ may be used (ρ_k = characteristic density in kg/m³; max. 500 kg/m³).

The design may be carried out as for nails in solid timber according to DIN EN 1995-1-1.

Axially loaded nails in the narrow faces may only be used when they are not placed in end grain.

Binderholz Brettsperrholz BBS	
Billide Holz Brettsper Holz BB3	
Fasteners	Annex 5



2.4 Screws

General

The outer thread diameter shall be used as the relevant diameter d of the screw. Penetration lengths $lef < 4 \cdot d$ should not be considered as load-carrying.

Laterally loaded screws - wide faces

The load direction must be perpendicular to the screw axis and parallel to the wide face of the cross laminated timber.

The minimum diameter for screws in the wide faces of Binderholz Brettsperrholz is 4.0 mm.

The embedding strength may be determined as for nails in solid timber according to DIN EN 1995-1-1 where the characteristic density of the cover layers is to be used.

For self-tapping screws arranged at an angle α of 30° $\leq \alpha <$ 90° with respect to the direction of grain of the cover layer the embedding strength shall be calculated as follows:

$$\begin{split} f_{h,\alpha,k} &= \frac{0.082 \cdot \rho_k \ d^{-0.3}}{2.5 \cdot \cos^2 \alpha + \sin^2 \alpha} & \text{in N/mm}^2 \qquad \text{without predrilled holes} \\ f_{h,\alpha,k} &= \frac{0.082 \cdot \rho_k \left(1 - 0.01 \cdot d\right)}{2.5 \cdot \cos^2 \alpha + \sin^2 \alpha} & \text{in N/mm}^2 \qquad \text{with predrilled holes} \end{split}$$

The effective number of screws n_{ef} may be set equal to the actual number n.

Laterally loaded screws - narrow faces

The load direction must be perpendicular to the screw axis and parallel to the narrow face of the cross laminated timber.

The minimum diameter for screws in the narrow faces of Binderholz Brettsperrholz is 8.0 mm.

Regardless of the arrangement of the screw in the narrow face (e.g. for angles between screw axis and grain direction of $0^{\circ} \le \alpha < 90^{\circ}$), the characteristic value of the embedding strength, when using screws without predrilling, shall be calculated as follows:

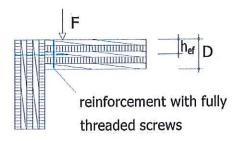
$$f_{h,k} = 20 \cdot d^{-0,5} \hspace{1cm} \text{in N/mm}^2$$

where

d nominal diameter of the screw in mm

The effective number of screws n_{ef} may be set as for bolts in solid timber according to DIN EN 1995-1-1.

Note: For actions perpendicular to the plane of the cross laminated timber the possibility of splitting caused by the tension force component perpendicular to the grain, shall be taken into account. Connections with ratios $h_{ef}/D \le 0.7$ should be reinforced with fully threaded screws (see Figure).



Binderholz Brettsperrholz BBS	
Fasteners	Annex 5



Axially loaded screws (pull-out)

The minimum value of the angle α between the screw axis and the grain direction is to be observed in accordance with the European Technical Approval or Assessment of the screw used.

The characteristic load-carrying capacity of an axially loaded screw is:

$$F_{ax,Rk} = \sum_{i=1}^{n} F_{ax,i,Rk} \qquad \qquad \text{in N}$$

where

F_{ax,i,Rk} characteristic withdrawal capacity for screws acc. to the European Technical Approval or Assessment in the board layer i depending on the characteristic density, the angle between the screw axis and grain direction, and the length of the threaded area of the screw in the board layer i

n number of board layers

Screws oriented parallel to the wide face of the cross laminated timber should be completely arranged within one board layer. The outer diameter of the threaded part should not exceed the thickness of the board layer the screw is arranged in.

The characteristic pull-through strength of the screw head is to be determined as for solid timber, depending on the characteristic density of the corresponding layer in the head area of the screw.

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Axially loaded screws (compressive capacity)

The characteristic value of the compressive load carrying capacity of a fully thread screw embedded in timber is the minimum of the axial resistance against pushing-in and the buckling resistance of the screw.

$$F_{ax,c,Rk} = min \begin{cases} F_{ax,Rk} \\ \\ \kappa_c \cdot N_{pl,k} \end{cases} \quad \text{in N} \label{eq:fax_Rk}$$

where

F_{ax,Rk} characteristic axial withdrawal capacity (see Axially loaded screws (pull-out))

$$\kappa_c = \begin{cases} 1 & \text{for } \overline{\lambda}_k \leq 0,2 \\ \\ \frac{1}{1/\!\left(k + \sqrt{\!\left(\!k^2 - \overline{\lambda}_k^2\right)}\right)} & \text{for } \overline{\lambda}_k > 0,2 \end{cases}$$

$$k \ = 0.5 \cdot \left[1 + 0.49 \left(\overline{\lambda}_k - 0.2 \right) + \overline{\lambda}_k^2 \right]$$

$$\overline{\lambda}_k = \sqrt{\frac{N_{pl,k}}{N_{ki,k}}}$$

$$N_{pl,k} = \pi \frac{d_2^2}{4} f_{y,k} \qquad \text{in N}$$

 d_2 = inner thread diameter of the screw in mm

 $f_{y,k}$ = characteristic yield strength in N/mm² acc. to the European Technical Approval or Assessment of the screw

 $N_{kl,k} = \sqrt{c_h E_S I_S}$ = characteristic elastic buckling load in N

 c_h = $(0.19 + 0.012 \, \text{d}) \rho_k \left(\frac{90^\circ + \alpha}{180^\circ}\right)$ = elastic foundation coefficient of the screw in N/mm2 for the most unfavorable combination of α and ρ_k

 ρ_k = characteristic density of a board layer

 α = angle between screw axis and grain direction of a board layer

 $E_{S} \cdot I_{S} = 210,000 \, \pi \, \frac{d_{2}^{4}}{64} = \text{bending stiffness of the inner thread diameter cross-section of the screw in Nmm²}$

2.5 Split ring, shear plate and toothed-plate connectors

The characteristic load-carrying capacity of split ring, shear plate and toothed-plate connectors in the wide faces of cross laminated timber may be calculated according to EN 1995-1-1 for an angle between force and grain direction of α = 0° regardless of the actual angle between the force and grain direction of the cover layers.

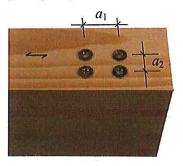
For split ring and shear plate connectors in the narrow faces of the cross laminated timber the regulations for connections with split ring connectors in the end grain of timber members may be applied.

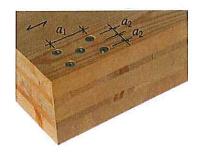
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Minimum spacings of fasteners

3.1 Minimum spacings of fasteners in the wide faces

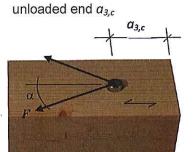
Minimum spacings - parallel and perpenticular to grain





Edge and end distances

loaded end $a_{3,t}$ $a_{3,t}$ α F



unloaded edge $a_{4,c}$ loaded edge $a_{4,t}$

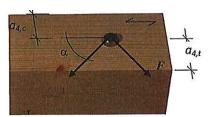


Table 4a: Minimum spacings of fasteners in the wide faces

fastener	a ₁	a _{3,t}	a _{3,c}	a ₂	a _{4,t}	a _{4,c}
screws 1)	4·d	6·d	6∙d	2.5·d	6·d	2.5·d
nails	(3+3·cosα)·d	(7+3·cosα)·d	6·d	3·d	(3+4·sinα)·d	3∙d
dowels	(3+2·cosα)·d	5·d	4·d·sinα min. 3·d	3·d	3·d	3∙d
bolts	(3+2·cosα)·d min. 4·d	5·d	4·d·sinα min. 4·d	4·d	3·d	3·d

 $\alpha \hspace{1cm} \text{angle between force and grain direction of the cover layer}$

self-tapping screws

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3.2 Minimum spacings, minimum thicknesses, minimum layer thicknesses und minimum penetration lengths of fasteners in the narrow faces

The minimum spacings in the narrow faces are independent of the angle between fastener axis and grain direction.

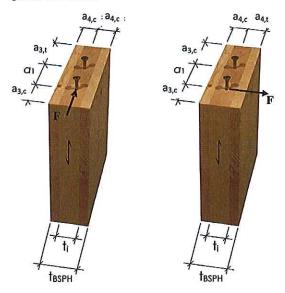


Table 4b: Minimum spacings of fasteners in the narrow faces

	a ₁	a _{3,t}	a _{3,c}	a ₂	a _{4,t}	a _{4,c}
screws 1)	10·d	12·d	7·d	3·d	6·d	5·d
dowels	4·d	5·d	3·d	3·d	5·d	3∙d
bolts	4·d	5·d	4·d	4·d	5·d	3·d

Table 4c: Requirements for fasteners in the narrow faces of cross laminated timber

fastener	Minimum thickness of the cross laminated timber	Minimum thickness of the relevant layer	Minimum penetration length of the fastener t_1 oder t_2 *)	
	t _{BSPH} in mm	t _i in mm	in mm	
screws	10·d	d > 8 mm: 3·d d ≤ 8 mm: 2·d	10·d	
dowels	6·d	d	5·d	

^{*)} t₁ Minimum penetration length of the fastener in side members (member to be connected)

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t₂ Minimum penetration length of the fastener in middle members (cross laminated timber element)



Reference documents

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Annex 6
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